



1.0 INPUT

1.1 Dimensions

Height of Staircase	H	=	1600 mm
No of Risers in Flight	N _{step}	=	10 Nos
Rise	R	=	160 mm
Going	T	=	250 mm
Length of Upper Landing	L _u	=	1000 mm
Length of Lower Landing	L _l	=	1000 mm

1.2 Sizes

Waist Slab Thickness	h _{span}	=	200 mm
Upper Landing Thickness	h _u	=	200 mm
Lower Landing Thickness	h _l	=	200 mm

1.3 Support Condition

Upper Landing Support Condition	S _{cu}	=	Simple
Lower Landing Support Condition	S _{cl}	=	Simple

1.4 Loads (Unfactored)

Characteristic Imposed Load	Q _k	=	4 kN/m ²
Characteristic Loads Due to Finishes	G _k	=	1 kN/m ²

1.4.1 Design Load Factors

Imposed Load Factor	Y _{fq}	=	1.6
Dead Load Factor	Y _{fg}	=	1.4

1.5 Reinforcement Details

Waist Slab		D 10 @ 110 mm C/C	
Upper Landing		D 10 @ 120 mm C/C	
Lower Landing		D 10 @ 150 mm C/C	
Clear Cover for Reinforcement	C _c	=	25 mm

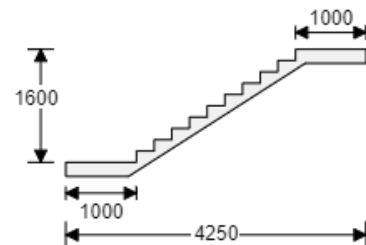
1.6 Materials

Concrete Grade	f _{cu}	=	35 N/mm ²
Main Reinforcement Grade	f _y	=	460 N/mm ²
Density of Concrete	Y _{conc}	=	24 kN/m ³

1.7 Deflection

Support Condition	SC	=	Simple
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Going (T) = 250 mm
Rise (R) = 160 mm



Allowable L/d Ratio $(L/d)_A = 20$

1.8 Material Safety Factors

Concrete in Compression $\gamma_{mc} = 1.5$
Concrete in Shear $\gamma_{mcs} = 1.25$
Reinforcement $\gamma_{ms} = 1.15$

2.0 OUTPUT

Ref: BS 8110 - Part 1

2.1 Design Load

Length of Staircase $L = (N_{\text{step}} - 1) * T + L_u + L_l = 4250 \text{ mm}$
Angle of Staircase $\Theta = \text{atan}(R / T) = 32.6 \text{ deg}$
Self-Weight of Staircase $q_{\text{kswt}} = [(h_{\text{span}} / T) + (\text{Rise} / 2)] * \gamma_{\text{conc}} = 7.6 \text{ kN/m}^2$
Design Load $F = \gamma_{\text{fig}} * (q_{\text{kswt}} + G_k) + \gamma_{\text{fq}} * Q_k = 18.5 \text{ kN/m}^2$ *Cl.3.2.1.2.2*

2.2 Waist Slab

Moment Coefficient $\beta = 0.086$ *Table 3.12*
Design Moment /m $M_{\text{span}} = \beta * F * L^2 = 28.7 \text{ kNm}$
Depth of Reinforcement $d = 170 \text{ mm}$
K Factor $K = M_{\text{span}} / (b * d^2 * f_{\text{cu}}) = 0.028$ *Cl.3.4.4.4*
K' Factor $K' = 0.2336 / \gamma_{\text{mc}} = 0.156$
Lever Arm $z = d * \min((0.5 + (0.25 - K / 0.9)^{0.5}), 0.95) = 161.5 \text{ mm}$
Area of Reinforcement Required /m $A_{s,\text{span}} = M_{\text{span}} / (f_y / \gamma_{\text{ms}} * z) = 444 \text{ mm}^2$
Area of Reinforcement Provided /m $A_{\text{ss,p}} = 714.3 \text{ mm}^2$

2.2.1 Deflection Check

Design Service Stress $f_s = (2 * f_y * A_{s,\text{span}}) / (3 * A_{\text{ss,p}}) = 190.6 \text{ N/mm}^2$
Modification Factor $\text{MF} = \min(0.55 + \{[477 - f_s] / \{120 * [0.9 + (M_{\text{span}} / d^2)]\}\}, 2) = 1.81$ *Cl.3.4.6.5*
Allowable Span/Effective Depth Ratio $(L/d)_{\text{Allow}} = \text{MF} * (L/d)_A = 36.2$ *Table 3.9*
Actual Span/Effective Depth Ratio $(L/d)_{\text{Actual}} = (L/d)_A = 25$

2.3 Upper Landing

Depth of Reinforcement $d = 170 \text{ mm}$
Area of Reinforcement Provided /m $A_{\text{su,p}} = 654.8 \text{ mm}^2$

2.3.1 Shear Check

Shear Coefficient $\alpha_u = 0.5$ *Table 3.12*
Design Shear Force $V_u = \alpha_u * F * L = 39.2 \text{ kN}$
Design Shear Stress $v = V_u / (b * d) = 0.231 \text{ N/mm}^2$ *Cl.3.4.5.2*
Percentage of Reinforcement $P_t = 0.39 \%$
Concrete Shear Stress $v_c = 0.637 \text{ N/mm}^2$ *Table 3.8*
Maximum Shear Stress $v_{\text{max}} = \min(0.8 * f_{\text{cu}}^{1/2}, 5) = 4.733 \text{ N/mm}^2$ *Cl.3.4.5.2*

2.4 Lower Landing

Depth of Reinforcement	d	= 170 mm
Area of Reinforcement Provided /m	$A_{sl,p}$	= 523.8 mm ²

2.4.1 Shear Check

Shear Coefficient	α_l	= 0.5
Design Shear Force	V_l	= $\alpha_l * F * L$ = 39.2 kN
Design Shear Stress	v	= $V_l / (b * d)$ = 0.231 N/mm ²
Percentage of Reinforcement	P_t	= 0.31 %
Concrete Shear Stress	v_c	= 0.591 N/mm ²

3.0 SUMMARY

3.1 Waist Slab

Description	Required	Actual	Status
Waist Slab Reinforcement (mm ² /m)	$A_{s,span}$ >= 444	$A_{ss,p}$ = 714.3	PASS
Minimum Area of Reinforcement (mm ² /m)	A_{min} >= 260	$A_{ss,p}$ = 714.3	PASS
Deflection	$(L/d)_{Allow}$ <= 36.2	$(L/d)_{Actual}$ = 25	PASS

3.2 Upper Landing

Description	Required	Actual	Status
Upper Landing Reinforcement (mm ² /m)	-	$A_{su,p}$ = 654.8	PASS
Minimum Area of Reinforcement (mm ² /m)	A_{min} >= 260	$A_{su,p}$ = 654.8	PASS
Shear Resistance (N/mm ²)	v >= 0.231	v_c = 0.637	PASS

3.3 Lower Landing

Description	Required	Actual	Status
Lower Landing Reinforcement (mm ² /m)	-	$A_{su,l}$ = 523.8	PASS
Minimum Area of Reinforcement (mm ² /m)	A_{min} >= 260	$A_{su,l}$ = 523.8	PASS
Shear Resistance (N/mm ²)	v >= 0.231	v_c = 0.591	PASS