



1.0 INPUT

1.1 Dimensions

Flange Type	Flange T
Section Width	$b = 300$ mm
Section Depth	$h = 450$ mm
Maximum Flange Width	$b_f = 600$ mm
Flange Depth	$h_f = 120$ mm
Distance between Zero Moments	$L = 1500$ mm

1.2 Reinforcement Cover

Clear Cover for Tension	$C_{ot} = 25$ mm
Clear Cover for Compression	$C_{oc} = 25$ mm
Side Cover	$C_{os} = 25$ mm

1.3 Design Loads

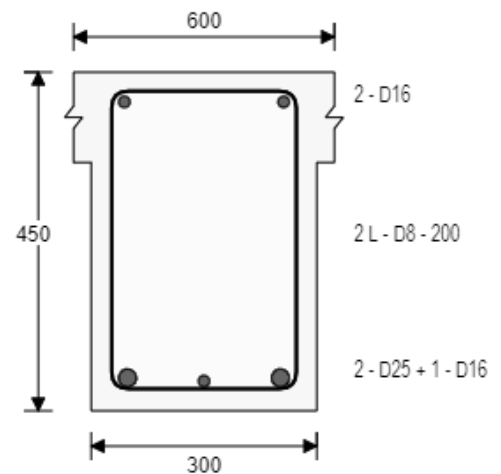
Design Moment	$M = 140$ kNm
Design Shear Force	$V = 0$ kN

1.4 Materials

Concrete Grade	$f_{cu} = 35$ N/mm ²
Main Reinforcement Grade	$f_y = 460$ N/mm ²
Shear Reinforcement Grade	$f_{yv} = 460$ N/mm ²
Aggregate Size	$h_{agg} = 20$ mm

1.4.1 Material Safety Factors

Concrete in Compression	$\gamma_{mc} = 1.5$
Concrete in Shear	$\gamma_{mcs} = 1.25$
Reinforcement	$\gamma_{ms} = 1.15$



2.0 OUTPUT

Ref: BS 8110 - Part 1

2.1 Flexure Design (Positive)

2.1.1 Tension Layer Effective Depth Calculation

2.1.1.1 Layer: 01

Rebars		2 - D 25 + 1 - D 16
Area of Reinforcement Provided	A_{t1}	= 1182.8 mm²
Depth of Layer 01	d_{t1}	= 404.5 mm
Total Area of Tension Steel Provided	A_{sbp}	= 1182.8 mm²
Effective Depth for Tension	d	= 404.5 mm

2.1.2 Compression Layer Effective Depth Calculation

2.1.2.1 Layer: 01

Rebars		2 - D 16	
Area of Reinforcement Provided	A_{c1}	= 402.1 mm²	
Depth of Layer 01	d_{c1}	= 41 mm	
Total Area of Compression Steel Provided	A_{stp}	= 402.1 mm²	
Effective Depth for Compression	d_2	= 41 mm	
Effective Width of Flange	b_{eff}	= $\min(b + L / 5, b_f) = \mathbf{600 \text{ mm}}$	<i>Cl. 3.4.1.5</i>
Flange Moment of Resistance	M_{Flange}	= $(0.67 * f_{cu} * b_{eff} * h_f * (d - 0.5 * h_f)) / \gamma_{mc}$ = 387.8 kNm	
Maximum Moment of Resistance	M_{max}	= $[(0.67 * f_{cu} * (b_{eff} - b) * h_f * (d - 0.5 * h_f))$ + $(0.2336 * f_{cu} * b * d^2)] / \gamma_{mc} = \mathbf{461.4 \text{ kNm}}$	
K Factor	K	= $M / (b * d^2 * f_{cu}) = \mathbf{0.041}$	<i>Cl. 3.4.4.4</i>
K' Factor	K'	= $0.2336 / \gamma_{mc} = \mathbf{0.156}$	
Lever Arm Distance	z	= $d * \min([0.5 + (0.25 - K / 0.9)^{1/2}], 0.95) = \mathbf{384.3 \text{ mm}}$	
Area of Tension Reinforcement	A_{smom}	= $M / (f_y * z / \gamma_{ms}) = \mathbf{910.8 \text{ mm}^2}$	<i>Cl. 3.4.4.4</i>
Minimum Area of Reinforcement	A_{min}	= $0.26\% * b * h = \mathbf{351 \text{ mm}^2}$	
Maximum Area of Reinforcement	A_{max}	= $4\% * b * h = \mathbf{5400 \text{ mm}^2}$	

3.0 SUMMARY

Description	Required	Actual	Status
Area of Tension Reinforcement (mm ²)	$A_{smom} \geq \mathbf{910.8}$	$A_{stp} = \mathbf{1182.8}$	PASS
Minimum Area of Reinforcement (mm ²)	$A_{min} \geq \mathbf{351}$	$A_{scp} = \mathbf{402.1}$	PASS
Maximum Area of Reinforcement (mm ²)	$A_{max} \leq \mathbf{5400}$	$A_{stp} = \mathbf{1182.8}$	PASS