



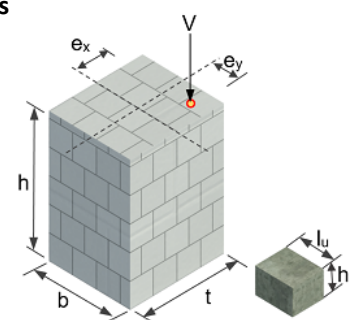
1.0 INPUT

1.1 Geometry

Height	h	=	3000 mm
Width	b	=	500 mm
Thickness	t	=	400 mm

1.2 Masonry Properties

Type	Aggregate Concrete Blocks 25% or less formed voids		
Compressive Strength of Unit	p_u	=	2.9 N/mm ²
Mortar Designation	M	=	M12/(i)
Height of Unit	h_u	=	200 mm
Least Horizontal Dimension of Unit	l_u	=	100 mm



1.3 Masonry Construction

Masonry Unit Category	MC	=	Category I
Construction Control Category	CC	=	Normal

1.4 Design Loads

Applied Factored Vertical Load	V	=	25 kN
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1.4.1 Eccentricity

Load Eccentricity along Thickness	e_x	=	20 mm
Load Eccentricity along Width	e_y	=	20 mm

2.0 OUTPUT

Ref: BS 5628 - Part 1

Characteristic Compressive Strength	f_k	=	2.8 N/mm ²	Table 2
Partial Safety Factor for Compression	γ_m	=	3.1	Table 4

2.1 Slenderness Check

Column Slenderness – Thickness	λ_x	=	$h / t =$ 7.5	Cl. 24.1
Column Slenderness – Width	λ_y	=	$h / b =$ 6	
Maximum Slenderness	λ_{max}	=	$\max(\lambda_x, \lambda_y) =$ 7.5	
Allowable Slenderness	λ_{allow}	=	27	

2.2 Capacity Reduction Factor

Additional Eccentricity - Thickness	e_{ax}	=	$\max(0, t * (((h / t)^2 / 2400) - 0.015)) =$ 3.4 mm	Annex B
Design Eccentricity - Thickness	e_{mx}	=	$\max(\text{abs}(e_x), (0.6 * \max(\text{abs}(e_x), 0.05 * t) + e_{ax})) =$ 20 mm	
Additional Eccentricity - Width	e_{ay}	=	$\max(0, b * (((h / b)^2 / 2400) - 0.015)) =$ 0 mm	

Design Eccentricity - Width	$e_{my} = \max(\text{abs}(e_y), (0.6 * \max(\text{abs}(e_y), 0.05 * b) + e_{ay})) = \mathbf{20 \text{ mm}}$
Capacity Reduction Factor - Thickness	$\beta_x = \max(0, 1.1 * (1 - (2 * e_{mx} / t))) = \mathbf{0.99}$
Capacity Reduction Factor - Width	$\beta_y = \max(0, 1.1 * (1 - (2 * e_{my} / b))) = \mathbf{1.012}$
Design Capacity Reduction Factor	$\beta = \min(1, \beta_x, \beta_y) = \mathbf{0.99}$

2.3 Design Vertical Load Resistance

Plan Area of Column	$A = t * b = \mathbf{0.2 \text{ m}^2}$	
Compressive Strength Correction Factor	$c = \min(1.0, 0.7 + (1.5 * A)) = \mathbf{1}$	<i>Cl. 19.1.2</i>
Design Vertical Load Resistance	$DVLR = \beta * t * b * c * f_k / \gamma_m = \mathbf{178.8 \text{ kN}}$	<i>Cl. 28.2.3</i>
Applied Stress	$v_s = V / (t * b) = \mathbf{0.125 \text{ N/mm}^2}$	
Allowable Stress	$v_a = DVLR / (t * b) = \mathbf{0.894 \text{ N/mm}^2}$	

3.0 SUMMARY

Description	Required	Actual	Status
Slenderness Limit	$\lambda_{allow} \leq \mathbf{27}$	$\lambda_{max} = \mathbf{7.5}$	PASS
Allowable Load (kN)	$V \geq \mathbf{25}$	$DVLR = \mathbf{178.8}$	PASS
Allowable Stress (N/mm ²)	$v_s \geq \mathbf{0.125}$	$v_a = \mathbf{0.894}$	PASS