

#### ECBEARING Masonry Bearing Wall Design

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## **1.0 INPUT**

#### 1.1 Masonry Wall

Wall Type		Solid
Load Bearing Leaf Thickness	t	= <b>100</b> mm
Height	h	= <b>2400</b> mm
Effective Height	$h_{eff}$	= <b>2400</b> mm

## 1.2 Pier

Width	W <sub>p</sub>	= 1	<b>00</b> mm
Thickness	tp	= 1	<b>00</b> mm
Spacing	Ws	= 1	<b>000</b> mm

#### 1.3 Masonry Properties

Туре	Clay ar	nd Calcium silicate bricks
Compressive Strength	p <sub>u</sub>	= <b>30</b> N/mm <sup>2</sup>
Mortar Designation	М	= M12/(i)

#### 1.4 Masonry construction

Masonry Unit category	MC	= Category I
Construction Control Category	CC	= Normal

## 1.5 Bearing

Beam Spanning		Across Wall
Edge Distance	$b_{ed}$	= <b>0</b> mm
Width	W <sub>b</sub>	= <b>100</b> mm
Length	L <sub>b</sub>	= <b>100</b> mm
Eccentricity at Top of Wall	e <sub>x</sub>	= <b>0</b> mm
		No

# 1.5.1 Spreader

1.6 Loads

#### 1.6.1 Concentrated Load

Characteristic Dead Load	G <sub>k</sub>	= <b>10</b> kN
Characteristic Imposed Load	Q <sub>k</sub>	= <b>8</b> kN
1.6.2 Distributed Load		
Characteristic Dead Load	g <sub>k</sub>	= <b>0</b> kN/m
Characteristic Imposed Load	q <sub>k</sub>	= <b>0</b> kN/m



# **2.0 OUTPUT**

## 2.1 Masonry Bearing Design

Stiffness Coefficient	К	= 1	cl.24.4.1
Effective Thickness of Masonry Wall	t <sub>eff</sub>	= <b>100</b> mm	cl.24.4
Characteristic Compressive Strength	$f_k$	= <b>8.3</b> N/mm <sup>2</sup>	Table 2
Partial Safety Factor for Material Strength	Υ <sub>m</sub>	= 3.1	Table 4

2.2 Design Loads

Design Concentrated Load	F	= ( $G_k * 1.4$ ) + ( $Q_k * 1.6$ ) = <b>26.8</b> kN
Design Distributed Load	f	= ( $g_k * 1.4$ ) + ( $q_k * 1.6$ ) = <b>0</b> kN/m

## 2.3 Bearing Check Without Spreader

Bearing Safety Factor	$\Upsilon_{\text{bear}}$	= 1.25	Cl. 30
Design Bearing Stress	$f_{cap}$	= F / ( W <sub>b</sub> * L <sub>b</sub> ) + f / t = <b>2.68</b> N/mm <sup>2</sup>	
Allowable Bearing Stress	f <sub>cpp</sub>	= $\Upsilon_{\text{bear}} * f_k / \Upsilon_m = 3.347 \text{ N/mm}^2$	

#### 2.3.1 Bearing check at 0.4 \* h below the level of bearing

Additional Eccentricity	e <sub>a</sub>	= t * ( (( h <sub>eff</sub> / t <sub>eff</sub> ) <sup>2</sup> / 2400 ) - 0.015 ) = <b>22.5</b> mm	
Eccentricity at Top of Wall	e <sub>x</sub>	= <b>0</b> mm	
Total Eccentricity	et	= ( 0.6 * e <sub>x</sub> ) + e <sub>a</sub> = <b>22.5</b> mm	
Design Eccentricity	e <sub>m</sub>	= max( e <sub>x</sub> , e <sub>t</sub> , 0.05 * t ) = <b>22.5</b> mm	
Capacity Reduction Factor	β	$= 1.1 * (1 - (2 * e_m / t)) = 0.61$ Table	27
Bearing Length Distributed at 0.4 * h	Ld	= W <sub>b</sub> + 0.4 * h + min( 0.4 * h, b <sub>ed</sub> ) = <b>1060</b> mm	
Design Bearing Stress	f <sub>ca(0.4h)</sub>	<sub>)</sub> = F / ( L <sub>d</sub> * t ) + f / t = <b>0.253</b> N/mm <sup>2</sup>	
Allowable Bearing Stress	f <sub>cp(0.4h</sub>	$_{\rm j} = \beta * f_{\rm k} / \Upsilon_{\rm m} = 1.62  \rm N/mm^2$	

## 3.0 SUMMARY

## 3.1 Bearing Check Without Spreader

Description	Required	Available	Status
Bearing stress (N/mm <sup>2</sup> )	f <sub>cap</sub> = <b>2.68</b>	f <sub>cpp</sub> = <b>3.347</b>	PASS

## 3.2 Bearing Check at 0.4 \* h Below the Level of Bearing

Description	Required		Available		Status
Bearing Stress (N/mm <sup>2</sup> )	f <sub>ca(0.4h)</sub>	=0.253	f <sub>cp(0.4h)</sub>	= 1.62	PASS