



1.0 INPUT

1.1 Dimensions

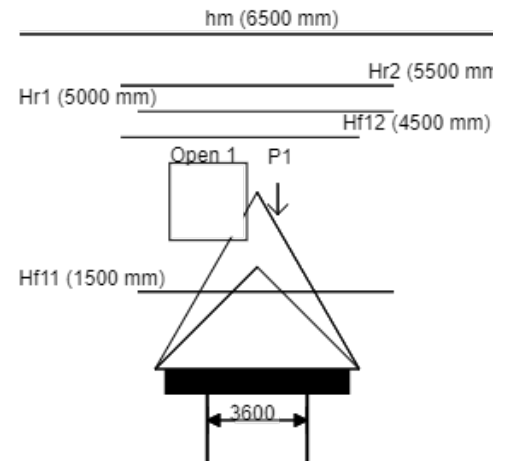
Height of Masonry	h_m	= 6500 mm
Leaf 1 Thickness	t_{m1}	= 100 mm
Clear Span of Lintel	L_c	= 3600 mm

1.2 Material Property

Masonry Type		Solid
Masonry Unit Weight Leaf 1	γ_{m1}	= 20 kN/m ³

1.3 Loads

Selfweight of Lintel	W_{s1}	= 0.15 kN/m
Dead Load Factor	γ_d	= 1.4
Imposed Load Factor	γ_i	= 1.6



Opening	Position (mm)		Dimensions (mm)	
	Along Lintel (OP_x)	Above Lintel (OP_y)	Width (W_{op})	Height (H_{op})
1	100	2500	1500	1500

Point Load	Position (mm)		Load (kN)	
	P_x	P_y	P_d	P_i
1	2200	3000	10	20

Floor Load 1	Height above Lintel (mm)	Dead Load (kN/m)	Imposed Load (kN/m)
	H_f	W_{fd}	W_{fi}
Side 1	1500	2	5
Side 2	4500	5	8

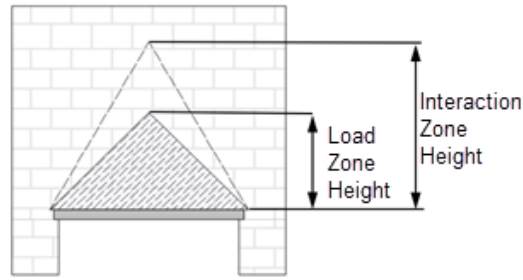
Roof Load	Height above Lintel (mm)	Dead Load (kN/m)	Imposed Load (kN/m)
	H_r	W_{rd}	W_{ri}
Side 1	5000	10	15
Side 2	5500	10	20

2.0 OUTPUT

Ref: BS 5977

2.1 Bearing Pressure Check

Lintel Load Application Length	L	= $L_c * 1.1 = 3960$ mm
Load Zone Height	h_{lz}	= $\tan(45^\circ) * L / 2 = 1980$ mm
Interaction Zone Height	h_{iz}	= $\tan(60^\circ) * L / 2 = 3429.5$ mm



2.1.1 Load of Masonry Dispersed on Lintel

Area of Load Zone	A_t	$= h_{iz} * L / 2 = 3.9 \text{ m}^2$
Masonry Load from Load Zone	W_{uvl}	$= A_t * (t_{m1} * \gamma_{m1} + t_{m2} * \gamma_{m2} + t_{mc} * \gamma_{mi}) = 7.8 \text{ kN}$
Equ. Masonry Load from Load Zone	W_{euvl}	$= 1.33 * W_{uvl} * \gamma_d / L = 3.7 \text{ kN/m}$

2.1.2 Dimensions of Lowest Opening Interacting with the Interaction Zone (I.Z)

Position of Opening along Lintel	OP_x	$= 100 \text{ mm}$
Position of Opening above Lintel	OP_y	$= 2500 \text{ mm}$
Width of Opening	W_{op}	$= 1500 \text{ mm}$
Height of Opening	H_{op}	$= 1500 \text{ mm}$

2.1.3 Additional Weight of Masonry

Length of Load at the Level of Opening	L_o	$= 1073.2 \text{ mm}$
Length of Load in Load Zone	L_{ot}	$= 0 \text{ mm}$
Length of Load - Other Side I.Z.	$L_{oiother}$	$= 736.6 \text{ mm}$
Length of Load - Opening Side I.Z.	$L_{oiopening}$	$= 0 \text{ mm}$
Add. Area due to Interaction of Opening	A_a	$= 4.8 \text{ m}^2$
Additional Weight of Masonry	W_a	$= \gamma_d * A_a * (t_{m1} * \gamma_{m1} + t_{m2} * \gamma_{m2} + t_{mc} * \gamma_{mi}) = 13.5 \text{ kN}$

2.1.4 Equivalent Load due to Additional Weight of Masonry

Masonry Load as UDL at Opening Level	W_{madd}	$= W_a * 0.5 = 6.8 \text{ kN}$
Total Length of Spread	$UDL_{mspread}$	$= 2 * OP_y + L_{oiother} = 5736.6 \text{ mm}$
Total Load on Lintel	W_{total}	$= W_{madd} / UDL_{mspread} = 1.2 \text{ kN/m}$
Equivalent UDL	W_{me}	$= W_{total} = 1.2 \text{ kN/m}$

2.2 Equivalent Load due to Floor Load 1 Side 1

2.2.1 Floor Load on Load Zone

Distance of Floor from Load Zone Top	Y_{f11T}	$= h_{iz} - H_{f11} = 480 \text{ mm}$
Half the Length of Floor in Load Zone	X_{f11T}	$= Y_{f11T} * (L / 2) / h_{iz} = 480 \text{ mm}$
Length of Floor in Load Zone	$U_{DLf11spread1}$	$= (2 * H_{f11} + 2 * X_{f11T}) = 3960 \text{ mm}$
Total Load Acting at the Load Zone	F_{11T}	$= (W_{fd11} * \gamma_d + W_{fi11} * \gamma_i) * 2 * X_{f11T} = 10.4 \text{ kN}$
Load Distributed at the Level of Lintel	W_{f11T}	$= F_{11T} / U_{DLf11spread1} = 2.6 \text{ kN/m}$

2.2.2 Floor Load at the Interaction Zone (I.Z.)

Total Load Acting	F_{11}	$= 0.5 * (W_{fd11} * \gamma_d + W_{fi11} * \gamma_i) = 5.4 \text{ kN/m}$
Distance of Floor from I.Z. Top	Y_{f11i}	$= h_{iz} - H_{f11} = 1929.5 \text{ mm}$
Half the Length of Floor in Load and I.Z.	X_{f11i}	$= Y_{f11i} * (L / 2) / h_{iz} = 1114 \text{ mm}$
Length of Floor in per Side of the I.Z.	$X_{interf11}$	$= X_{f11i} - X_{f11T} = 634 \text{ mm}$
Total Length of Spread at Lintel Level	$UDL_{f11spread2}$	$= 2 * H_{f11} + X_{interf11} = 3634 \text{ mm}$
Total Load Acting at the I.Z.	F_{11i}	$= F_{11} * X_{interf11} = 3.4 \text{ kN}$

Assessed UDL at Lintel Level	W_{f11i}	$= F_{11i} / \text{UDL}_{f11\text{spread}2} = \mathbf{0.9 \text{ kN/m}}$
Length of Load on Lintel	$\text{UDL}_{\text{partf}11}$	$= \mathbf{3 \text{ mm}}$
Total Load on Lintel	W_{f11iP}	$= W_{f11i} * \text{UDL}_{\text{partf}11} = \mathbf{2.8 \text{ kN}}$
Ratio of UDL Length to Lintel	n	$= \mathbf{0.8}$
K Factor	K	$= \mathbf{1.2}$
Total Equ. Load due to Roof Load 1 Side 1	W_{ef11}	$= 2 * ((K * W_{f11iP}) / L) + W_{f11T} = \mathbf{4.3 \text{ kN/m}}$

Table 1

2.3 Equivalent Load due to Floor Load 1 Side 2

Length of Floor in Opening Zone	L_{of}	$= \mathbf{1486.6 \text{ mm}}$
Total Load Acting	F_{12}	$= (W_{fd12} * \gamma_d + W_{fi12} * \gamma_i) * L_{of} = \mathbf{29.4 \text{ kN}}$
Total Load Acting at the I.Z.	$F_{12/m}$	$= 0.5 * F_{12} / \text{UDL}_{\text{mspread}} = \mathbf{2.6 \text{ kN/m}}$
Total Equ. Load due to Roof Load 1 Side 2	W_{ef12}	$= F_{12/m} = \mathbf{2.6 \text{ kN/m}}$

2.4 Equivalent Load due to Roof Load Side 1

Length of Floor in Opening Zone	L_{of}	$= \mathbf{1486.6 \text{ mm}}$
Total Load Acting	P_{DL+IL1}	$= (W_{rd1} * \gamma_d + W_{ri1} * \gamma_i) * L_{of} = \mathbf{56.5 \text{ kN}}$
Total Load Acting at the I.Z.	$P_{DL+IL1/m}$	$= 0.5 * P_{DL+IL1} / \text{UDL}_{\text{mspread}} = \mathbf{4.9 \text{ kN/m}}$
Total Equ. Load due to Roof Load 1 Side 1	W_{er1}	$= P_{DL+IL1/m} = \mathbf{4.9 \text{ kN/m}}$

2.5 Equivalent Load due to Roof Load Side 2

Length of Floor in Opening Zone	L_{of}	$= \mathbf{1486.6 \text{ mm}}$
Total Load Acting	P_{DL+IL2}	$= (W_{rd2} * \gamma_d + W_{ri2} * \gamma_i) * L_{of} = \mathbf{68.4 \text{ kN}}$
Total Load Acting at the I.Z.	$P_{DL+IL2/m}$	$= 0.5 * P_{DL+IL2} / \text{UDL}_{\text{mspread}} = \mathbf{6 \text{ kN/m}}$
Total Equivalent Load due to Roof Load Side 2	W_{er2}	$= P_{DL+IL2/m} = \mathbf{6 \text{ kN/m}}$

2.6 Equivalent Load due to Point Load 1

Total Applied Load	P_{con1}	$= (P_{d1} * \gamma_d) + (P_{i1} * \gamma_i) = \mathbf{46 \text{ kN}}$
Total Load on Lintel	$P_{con1/m}$	$= P_{con1} / \text{UDL}_{\text{mspread}} = \mathbf{8 \text{ kN/m}}$
Total Equivalent Load due to Point Load 1	W_{ep1}	$= W_{ep1} = P_{con1/m} = \mathbf{8 \text{ kN/m}}$

2.7 Analysis Results

Total Equivalent Load (Floor, Roof, Point Load, Opening, Lintel Self Weight)	W_e	$= W_{euvl} + W_{sl} * \gamma_d + W_{ef11} + W_{ef12} + W_{er1} + W_{er2} + W_{ep1}$ $= \mathbf{30.8 \text{ kN/m}}$
Maximum Bending Moment	M_{max}	$= (W_e * L^2) / 8 = \mathbf{60.4 \text{ kN.m}}$
Maximum Shear Force	SF_{max}	$= (\gamma_d * W_{uvl} + (W_{ef11} + W_{ef12} + W_{er1} + W_{er2} + W_{ep1} + \gamma_d * W_{sl}) * L) / 2$ $= \mathbf{59.2 \text{ kN}}$

3.0 SUMMARY

Description	Notation	Results
Total Equivalent Factored Load (kN/m)	$W_e =$	30.8
Maximum Bending Moment (kNm)	$M_{max} =$	60.4
Maximum Shear Force (kN)	$SF_{max} =$	59.2