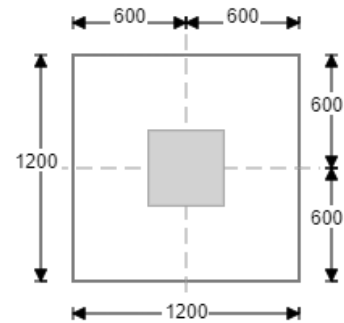




1.0 INPUT

1.1 Foundation

Analysis Options	Pedestal and Pad	
Depth of the Foundation below GL	D_f	= 1200 mm
Depth of the Water Table below GL	D_{wt}	= 350 mm
Extend of Minimum Soil Cover	S_{cve}	= Full
Minimum Soil Cover above Pad	S_{cv}	= 900 mm



1.2 Pad

Length of the Pad (X - Dir)	L_f	= 1200 mm
Breadth of the Pad (Z - Dir)	B_f	= 1200 mm
Thickness of the Pad	T_f	= 300 mm

1.3 Pedestal

Type of Pedestal	Pedestal	
Shape of Pedestal	Square	
Length (X - Dir)	L_c	= 400 mm
Breadth (Z - Dir)	B_c	= 400 mm
Top of Pedestal above GL	TOG	= 500 mm
Eccentricity (X - Dir)	E_x	= 0 mm
Eccentricity (Z - Dir)	E_z	= 0 mm

1.4 Reinforcement

1.4.1 Cover

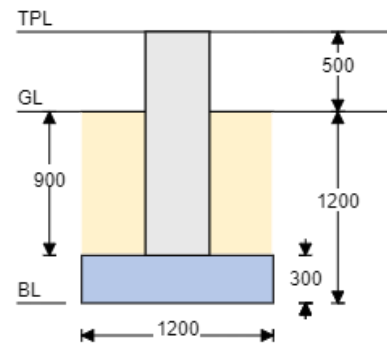
Clear Cover for Pedestal	C_{ped}	= 50 mm
Clear Cover for Pad	C_{pad}	= 50 mm

1.4.2 Pad - Bottom Reinforcement

Min % of Bottom Reinforcement	A_{bmin}	= 0.25 %
X - Direction		12 @ 150 mm C/C
Z - Direction		12 @ 150 mm C/C

1.4.3 Pad - Top Reinforcement

Min % of Top Reinforcement	A_{tmin}	= 0 %
X - Direction		N/A
Z - Direction		N/A



1.5 Pedestal

1.5.1 Main Reinforcement Details

Min % of Pedestal Reinforcement	A_{pedmin}	= 0.4 %
Corner Bars		# 4 - 16 mm
Depth Side Bars (Each face)		# 1 - 16 mm

Width Side Bars (Each face) # 1 - 16 mm

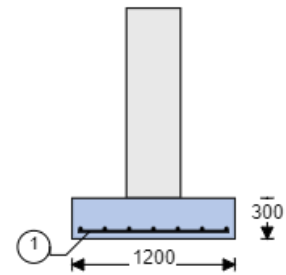
1.5.2 Ties

Shear Reinforcement Details Ties - 10 @ 150 mm C/C
 Additional Legs in X Direction $N_x = 0$
 Additional Legs in Z Direction $N_z = 0$

1.6 Design Loads

1.6.1 SLS

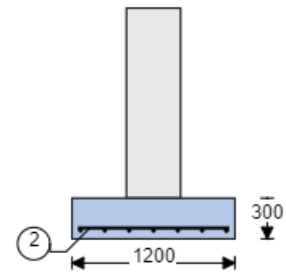
Load Factor for Self-Weight and Soil $F_{DS1} = 1$
 Axial Load $P_{S1} = 100 \text{ kN}$
 Horizontal Load (X - Dir) $F_{XS1} = 0 \text{ kN}$
 Horizontal Load (Z - Dir) $F_{ZS1} = 0 \text{ kN}$
 Moment about X Direction $M_{XS1} = 0 \text{ kN.m}$
 Moment about Z Direction $M_{ZS1} = 0 \text{ kN.m}$



X - Direction :
 1 : 12D @ 150 mm C/C

1.6.2 ULS

Load Factor for Self-Weight and Soil $F_{DU1} = 1.4$
 Axial Load $P_{U1} = 150 \text{ kN}$
 Horizontal Load (X - Dir) $F_{XU1} = 0 \text{ kN}$
 Horizontal Load (Z - Dir) $F_{ZU1} = 0 \text{ kN}$
 Moment about X Direction $M_{XU1} = 0 \text{ kN.m}$
 Moment about Z Direction $M_{ZU1} = 0 \text{ kN.m}$



Z - Direction :
 2 : 12D @ 150 mm C/C

1.7 Soil Properties

Allowable Net Bearing Pressure SBC = 150 kN/m³
 Density of Soil $\gamma_{soil} = 18.5 \text{ kN/m}^3$
 Density of Ground Water $\gamma_w = 9.81 \text{ kN/m}^3$
 Angle of Internal Friction $\Phi = 30 \text{ deg}$
 Coefficient of Friction $\mu = 0.35$

1.8 Concrete

Density of Concrete $\gamma_{conc} = 24 \text{ kN/m}^3$
 Unit Weight of Steel $\gamma_{steel} = 78.5 \text{ kN/m}^3$
 Concrete Grade @ 28 days $f_{cu} = 25 \text{ N/mm}^2$
 Main Reinforcement Grade $f_y = 500 \text{ N/mm}^2$
 Shear Reinforcement Grade $f_{yv} = 500 \text{ N/mm}^2$
 Modulus Elasticity of Steel $E_s = 200 \text{ kN/mm}^2$

1.9 Partial Safety Factors

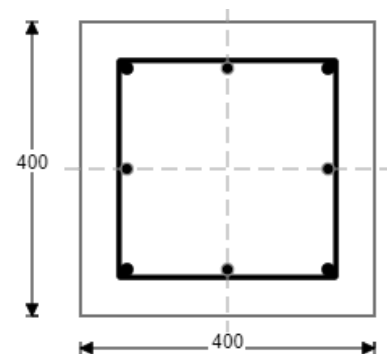
Concrete in Flexure/Compression $\gamma_{mc} = 1.5$
 Concrete in Shear $\gamma_{mcs} = 1.25$
 Reinforcement $\gamma_{ms} = 1.15$

1.10 Factor of Safety

F.O.S against Sliding $\gamma_{slide} = 1.5$
 F.O.S against Overturning $\gamma_{over} = 1.5$
 F.O.S against Uplift $\gamma_{up} = 1.2$

1.11 Crack Width

Check for Crack Width Yes



Rebars :
 (C) : #4 - 16 mm (W) : #1 - 16 mm
 (D) : #1 - 16 mm Ties : 10 mm @ 150 C/C

Allowable Crack Width

$$W_c = 0.2 \text{ mm}$$

2.0 OUTPUT

2.1 Weight Calculation

Ref: BS8110 - Part 1

Self Weight of Pad	Swf = 10.37 kN
Self Weight of Pedestals	Swpd = 5.38 kN
Minimum Soil Overburden	SIMnWt = 21.31 kN
Maximum Soil Overburden	SIMxWt = 21.31 kN
Water Weight above Pad	Wrfp = 6.91 kN
Deduction Weight of Suspended Soil	SIDnWt = 6.91 kN
Buoyancy Deduction Load	Bfuf = 12.01 kN
Total Weight under Min. Soil Condition	$M_{nCwt} = Swf + Swpd + Wrfp + SIMnWt - SIDnWt - Bfuf = 25.05 \text{ kN}$
Total Weight under Max. Soil Condition	$M_{xCwt} = Swf + Swpd + SIMxWt = 37.06 \text{ kN}$

2.2 Stability Calculations SLS Combination

Sliding Force	$S_{ldFor1} = \text{sqrt}(F_{XS1}^2 + F_{ZS1}^2) = 0 \text{ kN}$
Overturning Moment X Direction	$O_{vrMomX1} = M_{XS1} + F_{ZS1} * (H_{pd} + T_f) + P_{s1} * E_z = 0 \text{ kN.m}$
Overturning Moment Z Direction	$O_{vrMomZ1} = M_{ZS1} - F_{XS1} * (H_{pd} + T_f) - P_{s1} * E_z = 0 \text{ kN.m}$
Minimum Vertical Force	$M_{nVf1} = F_{DS1} * M_{nCwt} + P_{s1} = 125 \text{ kN}$
Minimum Weight Net Moment X Direction	$M_{nNetMomX1} = 0 \text{ kN.m}$
Minimum Weight Net Moment Z Direction	$M_{nNetMomZ1} = 0 \text{ kN.m}$
Maximum Vertical Force	$M_{xVf1} = F_{DS1} * M_{xCwt} + P_{s1} = 137.1 \text{ kN}$
Maximum Weight Net Moment X Direction	$M_{xNetMomX1} = 0 \text{ kN.m}$
Maximum Weight Net Moment Z Direction	$M_{xNetMomZ1} = 0 \text{ kN.m}$

2.3 Factor of Safety of SLS Combination

F.O.S. against Uplift	$F_{OSUPL1} = 100$
F.O.S. against Sliding	$F_{OSSLD1} = M_{nVf1} * \mu / S_{ldFor1} = 100$
F.O.S. against Overturning X Direction	$F_{OSOvX1} = 0.5 * M_{nVf1} * B_f / O_{vrMomX1} = 100$
F.O.S. against Overturning Z Direction	$F_{OSOvZ1} = 0.5 * M_{nVf1} * L_f / O_{vrMomZ1} = 100$

2.4 Base Pressure Calculation

Maximum Bearing Pressure	$M_{xBrPr1} = 95.2 \text{ kN/m}^2$
Percentage of Compression Area	$P_{erBaAr1} = 100 \%$

2.5 Crack Width

Ref: BS8110 - Part 2

2.5.1 X Direction

Cracking Moment @ Bottom	$M_{cr} = 5.706 \text{ kN.m}$
Width of Crack @ Bottom	$W_{cr} = 0 \text{ mm}$

2.5.2 Z Direction

Cracking Moment @ Bottom	$M_{cr} = 5.706 \text{ kN.m}$
Width of Crack @ Bottom	$W_{cr} = 0 \text{ mm}$

2.6 Strength Calculations ULS Combination

2.6.1 Area of Reinforcement Calculation X Direction

2.6.2 Bottom Reinforcement

Ultimate Design Moment /m	$M_{xBot} = 8.54$ kN.m
Required Area of Reinforcement	$A_{sxBot} = 390$ mm ²

2.6.3 One way Shear Check in X Direction

2.6.4 Positive Shear

Design Shear Force (d Distance)	$V_{xp} = 16.66$ kN
Design Shear Stress	$v = V_{xp} / (b * d) = 0.068$ N/mm ²
Percentage of Reinforcement	$P_t = 0.309$ %
Concrete Shear Stress	$v_c = 0.483$ N/mm ²

Table 3.8

2.7 Area of Reinforcement Calculation Z Direction

2.7.1 Bottom Reinforcement

Ultimate Design Moment /m	$M_{zBot} = 8.54$ kN.m
Required Area of Reinforcement	$A_{szBot} = 390$ mm ²

2.7.2 One way Shear Check in Z Direction

2.7.3 Positive Shear

Design Shear Force (d Distance)	$V_{zp} = 17.94$ kN
Design Shear Stress	$v = V_{zp} / (b * d) = 0.077$ N/mm ²
Percentage of Reinforcement	$P_t = 0.325$ %
Concrete Shear Stress	$v_c = 0.498$ N/mm ²

2.8 Punching Shear Check

2.8.1 Column Face

Design Shear Stress	$v = 0.359$ N/mm ²
Maximum Shear Stress	$v_{max} = 4$ N/mm ²

Cl. 3.7.6.4

2.8.2 1.5 d from Column Face

Design Shear Stress	$v = 0.02$ N/mm ²
Percentage of Reinforcement	$P_t = 0.264$ %
Concrete Shear Stress	$v_c = 0.462$ N/mm ²

2.8.3 Pedestal Design

Cl. 3.8.2

2.8.4 Main Reinforcement

Design Axial Load	$P = 157.5$ kN
Design Moment in X Direction	$M_x = 0$ kN.m
Design Moment in Z Direction	$M_z = 0$ kN.m
Required Area of Reinforcement	$A_s = 640$ mm ²

3.0 SUMMARY

3.1 Bearing Pressure Check

Load	Soil Pressure (kN/m ²)	Contact (%)
SLS	95.2 [172.2]	100

3.2 Stability Check

Load	Sliding	Overturning-X	Overturning-Z	Uplift
SLS	100 [1.5]	100 [1.5]	100 [1.5]	100 [1.2]

3.3 Crack Check (mm)

Load	Bottom - X Dir	Bottom - Z Dir	Top - X Dir	Top - Z Dir
SLS	0 [0.2]	0 [0.2]	0 [0.2]	0 [0.2]

3.4 Pad Reinforcement Check (mm²/m)

Load	Bottom - X Dir	Bottom - Z Dir	Top - X Dir	Top - Z Dir
ULS	390 [754]	390 [754]	0 [0]	0 [0]

3.5 Pad Shear Check (N/mm²)

Load	Bottom - X Dir	Bottom - Z Dir	Top - X Dir	Top - Z Dir
ULS	0.068 [0.483]	0.077 [0.498]	0 [0]	0 [0]

3.6 Punching Shear Check

Load	Pedestal	Punching Shear (N/mm ²)	Tension Face	Punching Distance (mm)
ULS	P1	0.359 [4]	Bottom	0
ULS	P1	0.02 [0.462]	Bottom	357

3.7 Pedestal Reinforcement Check

Load	Pedestal	Vertical rein. (mm ²)	Spacing (mm)
ULS	P1	640 [1608.4]	-